AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #3 Tamrack Ave/I-5 NB Ramps

Cycle (sec): 100 Critical Vol./Cap. (X): 0.929
Loss Time (sec): 9 (Y+R = 4 sec) Average Delay (sec/veh): 45.6
Optimal Cycle: 120 Level Of Service: D

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 1	0 0	1	0 0	0 0	0	1 0	2 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	234	1	667	0	0	0	120	609	0	0	448	172
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	234	1	667	0	0	0	120	609	0	0	448	172
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	260	1	741	0	0	0	133	677	0	0	498	191
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	260	1	741	0	0	0	133	677	0	0	498	191
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	260	1	741	0	0	0	133	677	0	0	498	191
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.85	0.85	0.85	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	0.91
Lanes:	0.99	0.01	1.00	0.00	0.00	0.00	1.00	2.00	0.00	0.00	1.45	0.55
Final Sat.:	1609	6	1615	0	0	0	1805	3610	0	0	2500	959
Capacity Analysis Module:												
Vol/Sat:	0.16	0.16	0.46	0.00	0.00	0.00	0.07	0.19	0.00	0.00	0.20	0.20
Crit Moves:			****					****				****
Green/Cycle:	0.49	0.49	0.49	0.00	0.00	0.00	0.20	0.20	0.00	0.00	0.21	0.21
Volume/Cap:	0.33	0.33	0.93	0.00	0.00	0.00	0.37	0.93	0.00	0.00	0.93	0.93
Delay/Veh:	15.5	15.5	40.8	0.0	0.0	0.0	35.0	57.5	0.0	0.0	56.6	56.6
User DelAdj:	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
AdjDel/Veh:	15.1	15.1	39.8	0.0	0.0	0.0	34.2	56.1	0.0	0.0	55.2	55.2
DesignQueue:	8	0	23	0	0	0	6	32	0	0	23	9

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #1 Tamarack Ave/Carlsbad Blvd

Cycle (sec):	100	Critical Vol./Cap. (X):	0.619
Loss Time (sec):	20 (Y+R = 4 sec)	Average Delay (sec/veh):	28.2
Optimal Cycle:	67	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	1 1	0	1 0	2 0	1	0 1	0 0	1	1 0	0 1	0
Volume Module:												
Base Vol:	35	240	45	55	861	51	26	41	34	273	65	72
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	35	240	45	55	861	51	26	41	34	273	65	72
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	39	267	50	61	957	57	29	46	38	303	72	80
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	39	267	50	61	957	57	29	46	38	303	72	80
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	39	267	50	61	957	57	29	46	38	303	72	80
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.98	0.98	0.85	0.95	0.92	0.92
Lanes:	1.00	1.68	0.32	1.00	2.00	1.00	0.39	0.61	1.00	1.00	0.47	0.53
Final Sat.:	1805	2968	556	1805	3610	1615	721	1143	1615	1805	829	921
Capacity Analysis Module:												
Vol/Sat:	0.02	0.09	0.09	0.03	0.27	0.04	0.04	0.04	0.02	0.17	0.09	0.09
Crit Moves:	****				****		****			****		
Green/Cycle:	0.03	0.34	0.34	0.13	0.43	0.43	0.07	0.07	0.07	0.27	0.27	0.27
Volume/Cap:	0.62	0.27	0.27	0.27	0.62	0.08	0.62	0.62	0.36	0.62	0.32	0.32
Delay/Veh:	64.8	24.3	24.3	40.1	23.0	17.0	54.9	54.9	46.9	34.3	29.5	29.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	64.8	24.3	24.3	40.1	23.0	17.0	54.9	54.9	46.9	34.3	29.5	29.5
DesignQueue:	2	10	2	3	33	2	2	2	2	13	3	3

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Tamarack Ave/I-5 SB Ramps

Cycle (sec): 100 Critical Vol./Cap. (X): 0.741
Loss Time (sec): 9 (Y+R = 4 sec) Average Delay (sec/veh): 29.6
Optimal Cycle: 57 Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 0	0 0	0	0 1	0 0	1	0 0	2 0	1	1 0	2 0	0
Volume Module:												
Base Vol:	0	0	0	152	0	156	0	371	347	534	364	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	152	0	156	0	371	347	534	364	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	0	0	0	169	0	173	0	412	386	593	404	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	169	0	173	0	412	386	593	404	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	0	0	0	169	0	173	0	412	386	593	404	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.95	1.00	0.85	1.00	0.95	0.85	0.95	0.95	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	0	0	0	1809	0	1615	0	3610	1615	1805	3610	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.09	0.00	0.11	0.00	0.11	0.24	0.33	0.11	0.00
Crit Moves:						****			****	****		
Green/Cycle:	0.00	0.00	0.00	0.14	0.00	0.14	0.00	0.32	0.32	0.44	0.44	0.00
Volume/Cap:	0.00	0.00	0.00	0.65	0.00	0.74	0.00	0.35	0.74	0.74	0.25	0.00
Delay/Veh:	0.0	0.0	0.0	45.9	0.0	53.0	0.0	26.1	35.8	26.8	17.5	0.0
User DelAdj:	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
AdjDel/Veh:	0.0	0.0	0.0	45.0	0.0	51.9	0.0	25.6	35.1	26.3	17.2	0.0
DesignQueue:	0	0	0	8	0	8	0	16	15	20	13	0

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #3 Tamrack Ave/I-5 NB Ramps

Cycle (sec):	100	Critical Vol./Cap. (X):	0.786
Loss Time (sec):	9 (Y+R = 4 sec)	Average Delay (sec/veh):	33.9
Optimal Cycle:	65	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 1	0 0	1	0 0	0 0	0	1 0	2 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	110	0	363	0	0	0	205	314	0	0	762	293
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	110	0	363	0	0	0	205	314	0	0	762	293
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	122	0	403	0	0	0	228	349	0	0	847	326
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	122	0	403	0	0	0	228	349	0	0	847	326
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	122	0	403	0	0	0	228	349	0	0	847	326
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	0.91
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	1.00	2.00	0.00	0.00	1.44	0.56
Final Sat.:	1809	0	1615	0	0	0	1805	3610	0	0	2497	961
Capacity Analysis Module:												
Vol/Sat:	0.07	0.00	0.25	0.00	0.00	0.00	0.13	0.10	0.00	0.00	0.34	0.34
Crit Moves:			****				****					****
Green/Cycle:	0.32	0.00	0.32	0.00	0.00	0.00	0.16	0.16	0.00	0.00	0.43	0.43
Volume/Cap:	0.21	0.00	0.79	0.00	0.00	0.00	0.79	0.60	0.00	0.00	0.79	0.79
Delay/Veh:	25.2	0.0	38.9	0.0	0.0	0.0	53.5	40.8	0.0	0.0	27.3	27.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.2	0.0	38.9	0.0	0.0	0.0	53.5	40.8	0.0	0.0	27.3	27.3
DesignQueue:	5	0	16	0	0	0	11	17	0	0	29	11

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #1 Tamarack Ave/Carlsbad Blvd

Cycle (sec):	100	Critical Vol./Cap. (X):	0.817
Loss Time (sec):	20 (Y+R = 4 sec)	Average Delay (sec/veh):	30.3
Optimal Cycle:	98	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	1 1	0	1 0	2 0	1	0 1	0 0	1	1 0	0 1	0
Volume Module:												
Base Vol:	62	1147	208	92	457	75	41	69	52	105	66	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	62	1147	208	92	457	75	41	69	52	105	66	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	69	1274	231	102	508	83	46	77	58	117	73	108
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	69	1274	231	102	508	83	46	77	58	117	73	108
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	69	1274	231	102	508	83	46	77	58	117	73	108
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.98	0.98	0.85	0.95	0.91	0.91
Lanes:	1.00	1.69	0.31	1.00	2.00	1.00	0.37	0.63	1.00	1.00	0.40	0.60
Final Sat.:	1805	2986	541	1805	3610	1615	698	1168	1615	1805	697	1032
Capacity Analysis Module:												
Vol/Sat:	0.04	0.43	0.43	0.06	0.14	0.05	0.07	0.07	0.04	0.06	0.10	0.10
Crit Moves:		****		****				****			****	
Green/Cycle:	0.13	0.52	0.52	0.07	0.46	0.46	0.08	0.08	0.08	0.13	0.13	0.13
Volume/Cap:	0.30	0.82	0.82	0.82	0.30	0.11	0.82	0.82	0.45	0.51	0.82	0.82
Delay/Veh:	40.4	22.9	22.9	78.6	16.8	15.2	73.4	73.4	46.2	42.5	63.0	63.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.4	22.9	22.9	78.6	16.8	15.2	73.4	73.4	46.2	42.5	63.0	63.0
DesignQueue:	3	38	7	5	16	2	2	4	3	6	4	5

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Tamarack Ave/I-5 SB Ramps

Cycle (sec): 100 Critical Vol./Cap. (X): 0.549
Loss Time (sec): 9 (Y+R = 4 sec) Average Delay (sec/veh): 30.0
Optimal Cycle: 37 Level Of Service: C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 0	0 0	0	0 1	0 0	1	0 0	2 0	1	1 0	2 0	0
Volume Module:												
Base Vol:	0	0	0	245	0	248	0	470	229	237	461	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	245	0	248	0	470	229	237	461	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	0	0	0	272	0	276	0	522	254	263	512	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	272	0	276	0	522	254	263	512	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	0	0	0	272	0	276	0	522	254	263	512	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.95	1.00	0.74	1.00	0.95	0.85	0.95	0.95	1.00
Lanes:	0.00	0.00	0.00	1.00	0.00	1.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	0	0	0	1809	0	1405	0	3610	1615	1805	3610	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.15	0.00	0.20	0.00	0.14	0.16	0.15	0.14	0.00
Crit Moves:						****			****	****		
Green/Cycle:	0.00	0.00	0.00	0.36	0.00	0.36	0.00	0.29	0.29	0.27	0.27	0.00
Volume/Cap:	0.00	0.00	0.00	0.42	0.00	0.55	0.00	0.50	0.55	0.55	0.53	0.00
Delay/Veh:	0.0	0.0	0.0	24.7	0.0	26.9	0.0	30.2	31.6	32.9	32.0	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	24.7	0.0	26.9	0.0	30.2	31.6	32.9	32.0	0.0
DesignQueue:	0	0	0	10	0	10	0	21	10	11	22	0

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

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Intersection #3 Tamrack Ave/I-5 NB Ramps

****

Cycle (sec): 100 Critical Vol./Cap. (X): 0.929
Loss Time (sec): 9 (Y+R = 4 sec) Average Delay (sec/veh): 46.6
Optimal Cycle: 120 Level Of Service: D

****

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 1	0 0	1	0 0	0 0	0	1 0	2 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	235	0	667	0	0	0	135	609	0	0	448	172
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	235	0	667	0	0	0	135	609	0	0	448	172
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	261	0	741	0	0	0	150	677	0	0	498	191
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	261	0	741	0	0	0	150	677	0	0	498	191
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	261	0	741	0	0	0	150	677	0	0	498	191
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	1.00	0.85	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	0.91
Lanes:	1.00	0.00	1.00	0.00	0.00	0.00	1.00	2.00	0.00	0.00	1.45	0.55
Final Sat.:	1809	0	1615	0	0	0	1805	3610	0	0	2500	959
Capacity Analysis Module:												
Vol/Sat:	0.14	0.00	0.46	0.00	0.00	0.00	0.08	0.19	0.00	0.00	0.20	0.20
Crit Moves:			****					****				****
Green/Cycle:	0.49	0.00	0.49	0.00	0.00	0.00	0.20	0.20	0.00	0.00	0.21	0.21
Volume/Cap:	0.29	0.00	0.93	0.00	0.00	0.00	0.41	0.93	0.00	0.00	0.93	0.93
Delay/Veh:	15.2	0.0	40.8	0.0	0.0	0.0	35.5	57.5	0.0	0.0	56.6	56.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	15.2	0.0	40.8	0.0	0.0	0.0	35.5	57.5	0.0	0.0	56.6	56.6
DesignQueue:	8	0	23	0	0	0	7	32	0	0	23	9

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