APPENDIX C

SIGNALIZED INTERSECTION ANALYSIS SHEETS

AGUA HEDIONDA LAGOON

Scenario Report

Scenario:	Ex-AM
Command:	Default Command
Volume:	Ex-AM
Geometry:	Ex-AM
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Paths
Routes:	Default Routes
Configuration:	Default Configuration

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

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Intersection #1 Tamarack Ave/Carlsbad Blvd

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Cycle (sec):	100	Critical Vol./Cap. (X):	0.607
Loss Time (sec):	20 (Y+R = 4 sec)	Average Delay (sec/veh):	27.5
Optimal Cycle:	66	Level Of Service:	C

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	1 1	0	1 0	2 0	1	0 1	0 0	1	1 0	0 1	0
Volume Module:												
Base Vol:	35	240	45	55	861	51	26	26	34	273	50	72
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	35	240	45	55	861	51	26	26	34	273	50	72
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	39	267	50	61	957	57	29	29	38	303	56	80
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	39	267	50	61	957	57	29	29	38	303	56	80
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	39	267	50	61	957	57	29	29	38	303	56	80
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.98	0.98	0.85	0.95	0.91	0.91
Lanes:	1.00	1.68	0.32	1.00	2.00	1.00	0.50	0.50	1.00	1.00	0.41	0.59
Final Sat.:	1805	2968	556	1805	3610	1615	927	927	1615	1805	714	1019
Capacity Analysis Module:												
Vol/Sat:	0.02	0.09	0.09	0.03	0.27	0.04	0.03	0.03	0.02	0.17	0.08	0.08
Crit Moves:	****				****			****		****		
Green/Cycle:	0.04	0.34	0.34	0.13	0.44	0.44	0.05	0.05	0.05	0.28	0.28	0.28
Volume/Cap:	0.61	0.26	0.26	0.26	0.61	0.08	0.61	0.61	0.46	0.61	0.28	0.28
Delay/Veh:	63.1	23.8	23.8	39.9	22.3	16.5	57.2	57.2	50.0	33.6	28.7	28.7
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	63.1	23.8	23.8	39.9	22.3	16.5	57.2	57.2	50.0	33.6	28.7	28.7
DesignQueue:	2	10	2	3	32	2	2	2	2	13	2	3

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Tamarack Ave/I-5 SB Ramps

Cycle (sec):	100	Critical Vol./Cap. (X):	0.739
Loss Time (sec):	9 (Y+R = 4 sec)	Average Delay (sec/veh):	28.7
Optimal Cycle:	57	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 0	0 0	0	0 1	0 0	1	0 0	2 0	1	1 0	2 0	0
Volume Module:												
Base Vol:	0	0	0	151	1	141	0	356	347	534	364	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	151	1	141	0	356	347	534	364	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	0	0	0	168	1	157	0	396	386	593	404	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	168	1	157	0	396	386	593	404	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	0	0	0	168	1	157	0	396	386	593	404	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.85	0.85	0.85	1.00	0.95	0.85	0.95	0.95	1.00
Lanes:	0.00	0.00	0.00	0.99	0.01	1.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	0	0	0	1605	10	1615	0	3610	1615	1805	3610	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.10	0.10	0.10	0.00	0.11	0.24	0.33	0.11	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.14	0.14	0.14	0.00	0.32	0.32	0.44	0.44	0.00
Volume/Cap:	0.00	0.00	0.00	0.74	0.74	0.69	0.00	0.34	0.74	0.74	0.25	0.00
Delay/Veh:	0.0	0.0	0.0	53.1	53.1	49.2	0.0	25.9	35.6	26.6	17.4	0.0
User DelAdj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
AdjDel/Veh:	0.0	0.0	0.0	50.5	50.5	46.8	0.0	24.6	33.8	25.3	16.6	0.0
DesignQueue:	0	0	0	8	0	8	0	15	15	20	13	0

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AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

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Intersection #3 Tamrack Ave/I-5 NB Ramps

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Cycle (sec): 100 Critical Vol./Cap. (X): 0.775
Loss Time (sec): 9 (Y+R = 4 sec) Average Delay (sec/veh): 33.5
Optimal Cycle: 63 Level Of Service: C

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 1	0 0	1	0 0	0 0	0	1 0	2 0	0	0 0	1 1	0
Volume Module:												
Base Vol:	109	1	363	0	0	0	190	314	0	0	762	293
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	109	1	363	0	0	0	190	314	0	0	762	293
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	121	1	403	0	0	0	211	349	0	0	847	326
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	121	1	403	0	0	0	211	349	0	0	847	326
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	121	1	403	0	0	0	211	349	0	0	847	326
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.85	0.85	0.85	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.91	0.91
Lanes:	0.99	0.01	1.00	0.00	0.00	0.00	1.00	2.00	0.00	0.00	1.44	0.56
Final Sat.:	1602	13	1615	0	0	0	1805	3610	0	0	2497	961
Capacity Analysis Module:												
Vol/Sat:	0.08	0.08	0.25	0.00	0.00	0.00	0.12	0.10	0.00	0.00	0.34	0.34
Crit Moves:			****				****					****
Green/Cycle:	0.32	0.32	0.32	0.00	0.00	0.00	0.15	0.15	0.00	0.00	0.44	0.44
Volume/Cap:	0.23	0.23	0.78	0.00	0.00	0.00	0.78	0.64	0.00	0.00	0.78	0.78
Delay/Veh:	25.1	25.1	37.8	0.0	0.0	0.0	53.9	42.5	0.0	0.0	26.5	26.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	25.1	25.1	37.8	0.0	0.0	0.0	53.9	42.5	0.0	0.0	26.5	26.5
DesignQueue:	5	0	16	0	0	0	10	17	0	0	29	11

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AGUA HEDIONDA LAGOON

Scenario Report

Scenario:	Ex-PM
Command:	Default Command
Volume:	Ex-PM
Geometry:	Ex-AM
Impact Fee:	Default Impact Fee
Trip Generation:	Default Trip Generation
Trip Distribution:	Default Trip Distribution
Paths:	Default Paths
Routes:	Default Routes
Configuration:	Default Configuration

AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

Intersection #1 Tamarack Ave/Carlsbad Blvd

Cycle (sec):	100	Critical Vol./Cap. (X):	0.796
Loss Time (sec):	20 (Y+R = 4 sec)	Average Delay (sec/veh):	28.5
Optimal Cycle:	94	Level Of Service:	C

Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Protected			Protected			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	1 0	1 1	0	1 0	2 0	1	0 1	0 0	1	1 0	0 1	0
Volume Module:												
Base Vol:	62	1147	208	92	457	75	41	54	52	105	51	97
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	62	1147	208	92	457	75	41	54	52	105	51	97
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	69	1274	231	102	508	83	46	60	58	117	57	108
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	69	1274	231	102	508	83	46	60	58	117	57	108
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	69	1274	231	102	508	83	46	60	58	117	57	108
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.95	0.93	0.93	0.95	0.95	0.85	0.98	0.98	0.85	0.95	0.90	0.90
Lanes:	1.00	1.69	0.31	1.00	2.00	1.00	0.43	0.57	1.00	1.00	0.35	0.65
Final Sat.:	1805	2986	541	1805	3610	1615	807	1053	1615	1805	592	1122
Capacity Analysis Module:												
Vol/Sat:	0.04	0.43	0.43	0.06	0.14	0.05	0.06	0.06	0.04	0.06	0.10	0.10
Crit Moves:		****		****			****					****
Green/Cycle:	0.13	0.54	0.54	0.07	0.48	0.48	0.07	0.07	0.07	0.12	0.12	0.12
Volume/Cap:	0.29	0.80	0.80	0.80	0.29	0.11	0.80	0.80	0.50	0.54	0.80	0.80
Delay/Veh:	40.1	21.2	21.2	73.9	16.0	14.4	73.0	73.0	48.1	43.9	61.6	61.6
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	40.1	21.2	21.2	73.9	16.0	14.4	73.0	73.0	48.1	43.9	61.6	61.6
DesignQueue:	3	37	7	5	15	2	2	3	3	6	3	5

AGUA HEDIONDA LAGOON

Level Of Service Computation Report
2000 HCM Operations Method (Base Volume Alternative)

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Intersection #2 Tamarack Ave/I-5 SB Ramps

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Cycle (sec): 100 Critical Vol./Cap. (X): 0.518
Loss Time (sec): 9 (Y+R = 4 sec) Average Delay (sec/veh): 29.4
Optimal Cycle: 35 Level Of Service: C

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Approach:	North Bound			South Bound			East Bound			West Bound		
Movement:	L	T	R	L	T	R	L	T	R	L	T	R
Control:	Split Phase			Split Phase			Split Phase			Split Phase		
Rights:	Include			Include			Include			Include		
Min. Green:	0	0	0	0	0	0	0	0	0	0	0	0
Lanes:	0 0	0 0	0	0 1	0 0	1	0 0	2 0	1	1 0	2 0	0
Volume Module:												
Base Vol:	0	0	0	244	1	233	0	455	229	237	461	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	0	0	244	1	233	0	455	229	237	461	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
PHF Volume:	0	0	0	271	1	259	0	506	254	263	512	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	0	0	0	271	1	259	0	506	254	263	512	0
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Final Vol.:	0	0	0	271	1	259	0	506	254	263	512	0
Saturation Flow Module:												
Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	1.00	1.00	1.00	0.85	0.85	0.85	1.00	0.95	0.85	0.95	0.95	1.00
Lanes:	0.00	0.00	0.00	0.99	0.01	1.00	0.00	2.00	1.00	1.00	2.00	0.00
Final Sat.:	0	0	0	1609	6	1615	0	3610	1615	1805	3610	0
Capacity Analysis Module:												
Vol/Sat:	0.00	0.00	0.00	0.17	0.17	0.16	0.00	0.14	0.16	0.15	0.14	0.00
Crit Moves:				****					****	****		
Green/Cycle:	0.00	0.00	0.00	0.33	0.33	0.33	0.00	0.30	0.30	0.28	0.28	0.00
Volume/Cap:	0.00	0.00	0.00	0.52	0.52	0.49	0.00	0.46	0.52	0.52	0.50	0.00
Delay/Veh:	0.0	0.0	0.0	28.3	28.3	27.9	0.0	28.5	29.8	31.2	30.5	0.0
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	0.0	0.0	0.0	28.3	28.3	27.9	0.0	28.5	29.8	31.2	30.5	0.0
DesignQueue:	0	0	0	11	0	10	0	20	10	11	21	0

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